

# Proposed Seismic Detailing Criteria for Piers and Wharves

*Robert E. Harn, P.E., M.ASCE,  
Timothy W. Mays, Ph.D., P.E., M.ASCE,  
Gayle S. Johnson, P.E., M.ASCE*

# Overview

- Update on proposed seismic code
- Present goals of detailing provisions
- Review performance of several pile to deck connections
- Summarize spiral requirements

# Update on Proposed Seismic Code

- New ASCE standard, “**Seismic Design of Piers and Wharves**” under development for 4 years
- Codifies current practice of performance-based seismic design
- Same format and legal standing as ASCE 7
- Pile supported structures only
- Volunteer effort

# Proposed Table of Contents in Standard

Chapter 1 - General

Chapter 2 - Seismic Performance Requirements

Chapter 3 - Design Approach

Chapter 4 - Geotechnical Considerations

Chapter 5 - Force Based Analysis and Design

Chapter 6 - Displacement Based Analysis and Design

**Chapter 7 - Design and Detailing Considerations** ←

Chapter 8 – Ancillary Components

# Chapter 7 Table of Contents

- 7.1 Introduction
- 7.2 Definitions
- 7.3 Symbols and Notation
- 7.4 **Pile to Deck Connections** ←
- 7.5 **Confinement** ←
- 7.6 Joint Region Dowel Anchorage
- 7.7 Joint Shear
- 7.8 Joint Detailing
- 7.9 Decks
- 7.10 Constructibility

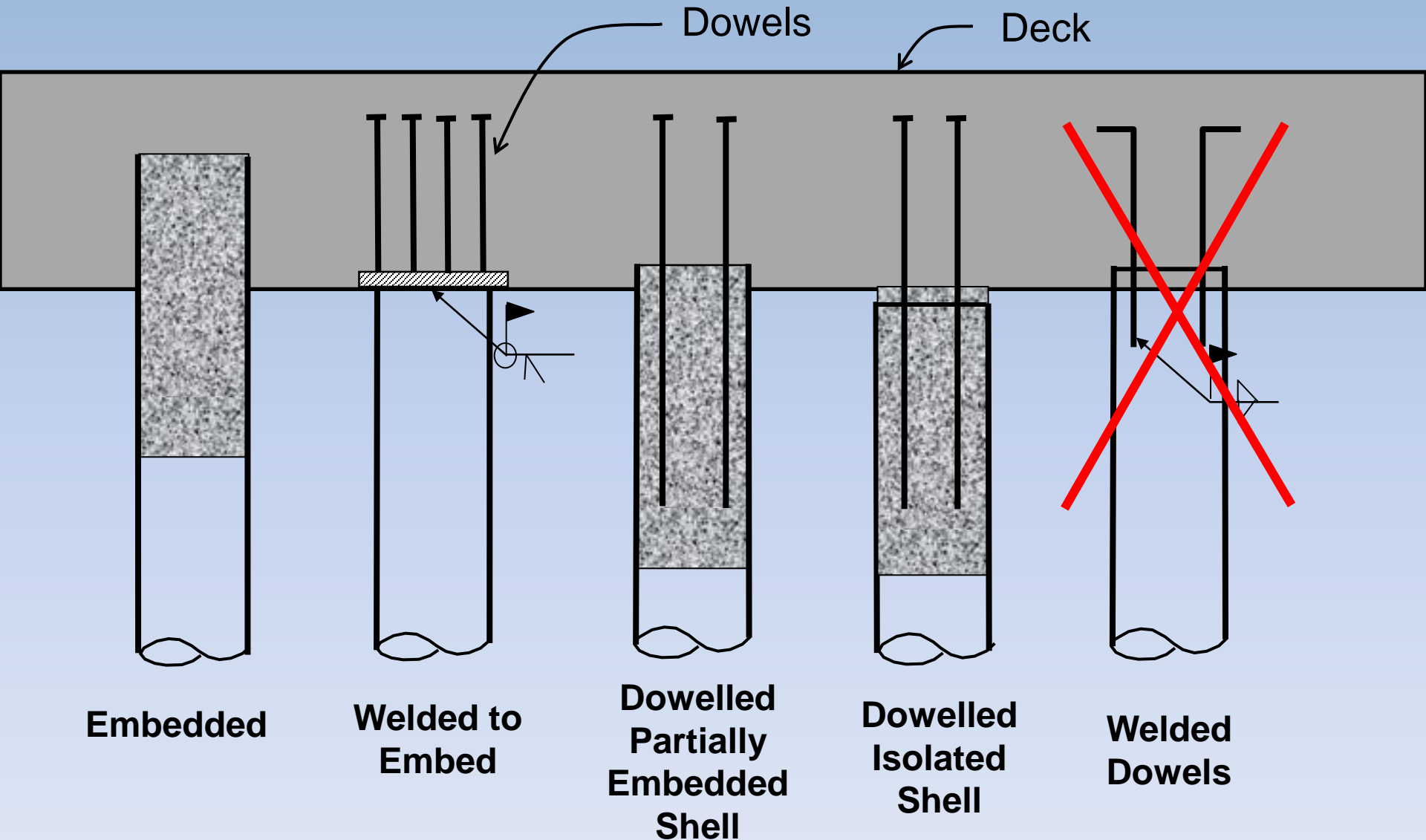
# Goals of Detailing Provisions

- Include all commonly used pile connections
  - Backed by testing
  - Ductile and suitable for seismic
- Include commonly used deck systems.
- Use existing codes
  - MOTEMS
  - POLA
  - POLB
- Address seismic detailing issues unique to piers and wharves
  - Pile driving tolerances
  - Pile cut-offs and build-ups

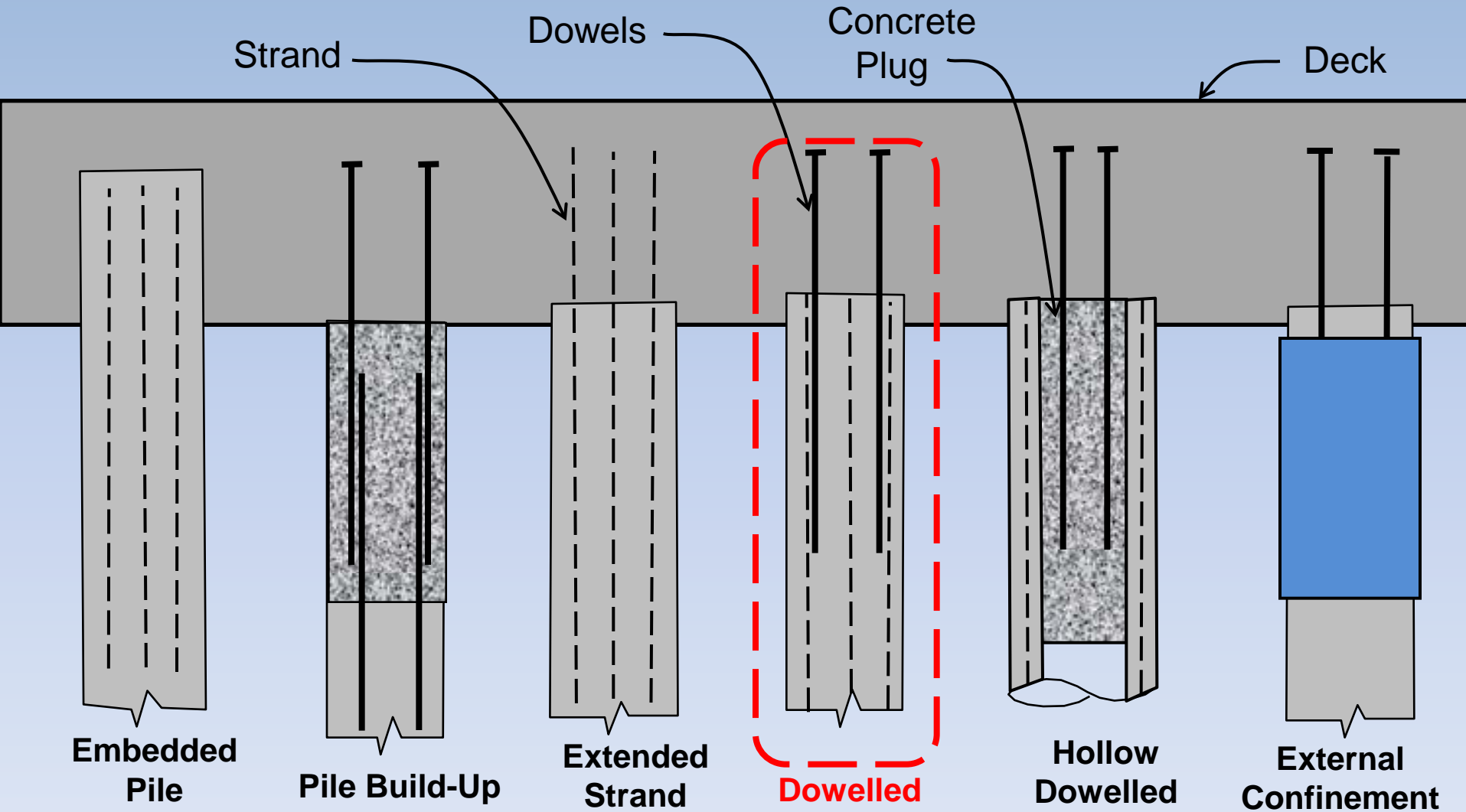
# Pile to Deck Connection Testing

- Significant recent research and test data for prestressed concrete piles
- Limited research and test data for pipe piles
- Presentation focus is on prestressed concrete piles

# Example Pipe Pile Connections



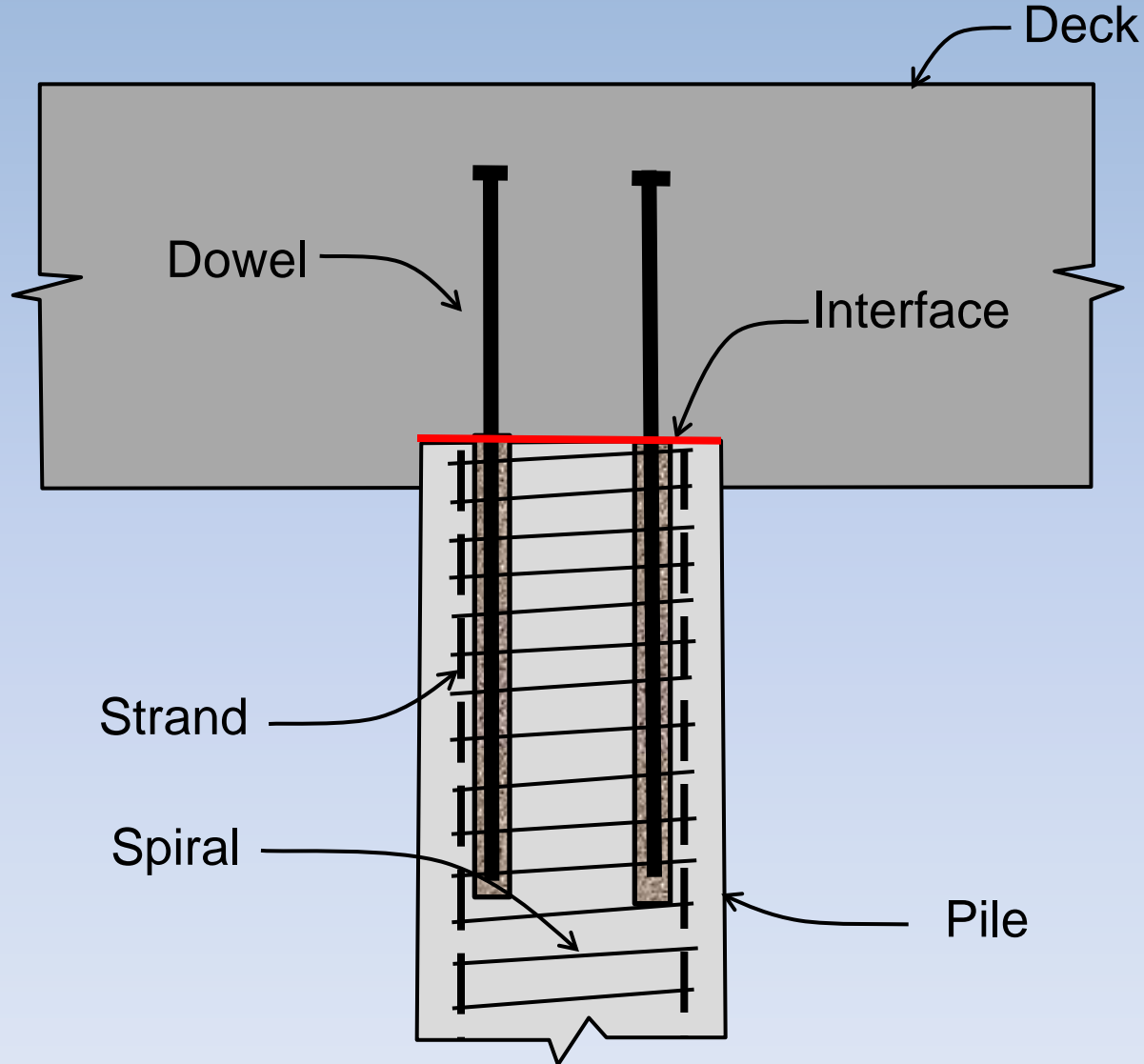
# Example Prestressed Concrete Pile Connections



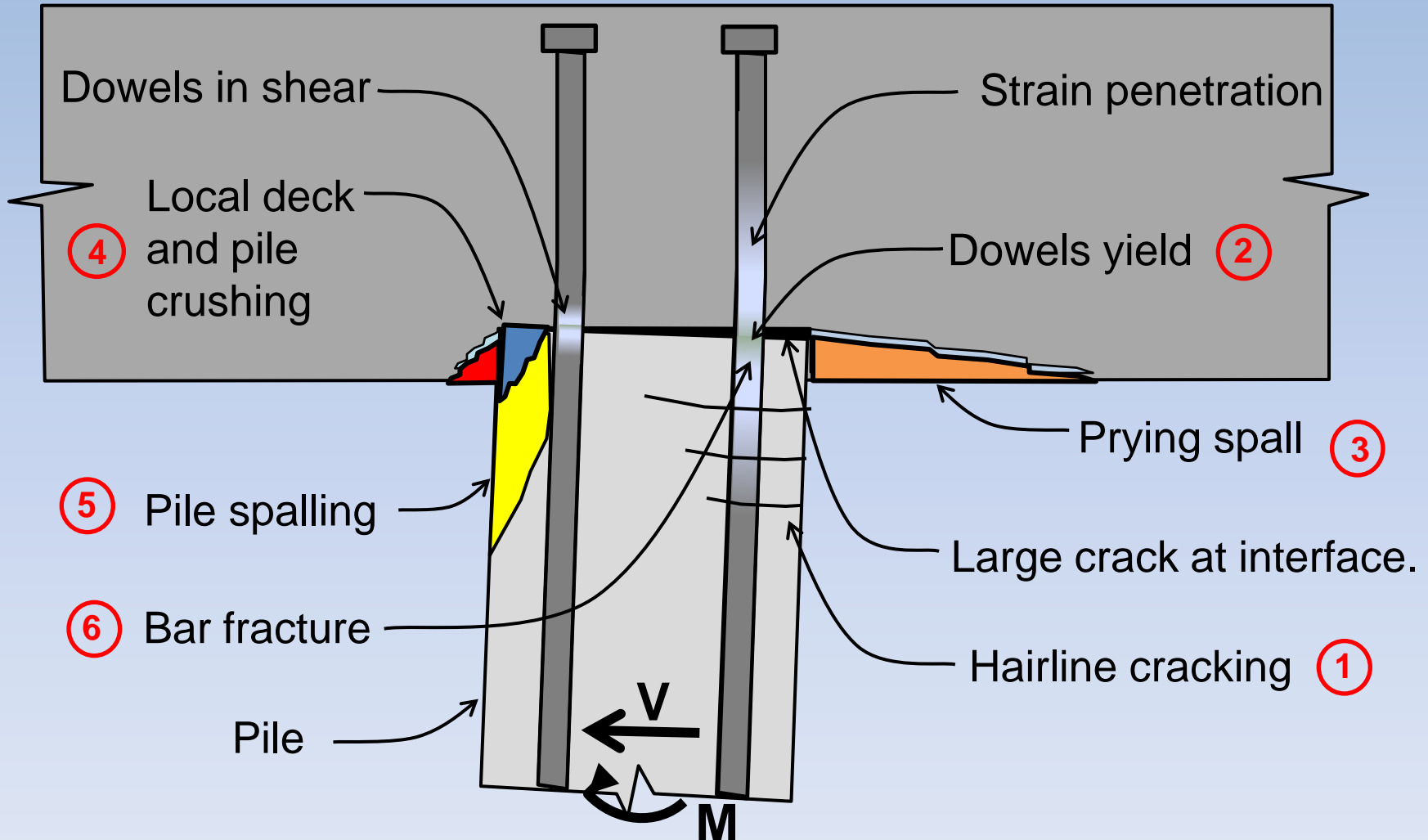
# Prestressed Concrete Dowelled Connection Test Overview

- Define terms
- Review typical behavior
  - Strong pile
  - Weak interface
- POLA pile tests
- UW pile tests
- Summary

# Pile to Deck Connection Terms

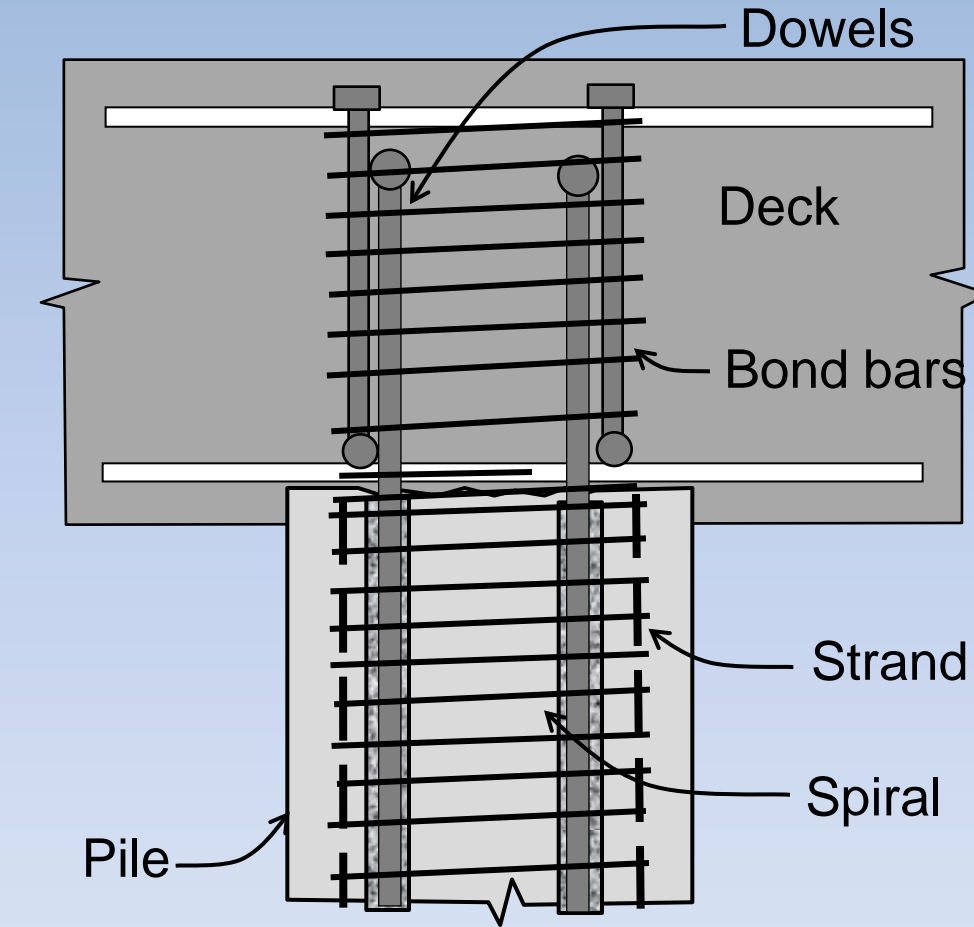


# Typical Behavior of a Prestressed Concrete Pile Dowelled Connection



# POLA Seismic Pile Tests (Ref. 2, 3)

- 36" deck
- 24" Octagonal Pile
- 16 - 0.6" dia. Strand
- 8 - #10 dowels
- W20 @ 2.5 Spiral



Embed 1 ½ spiral turns in deck

# POLA Seismic Pile Behavior (Ref. 2, 3)



Slab Prying  $\mu_{\Delta} = 1.5$  (1.2% drift)



Deck Crushing  $\mu_{\Delta} = 2$  (1.6% drift)

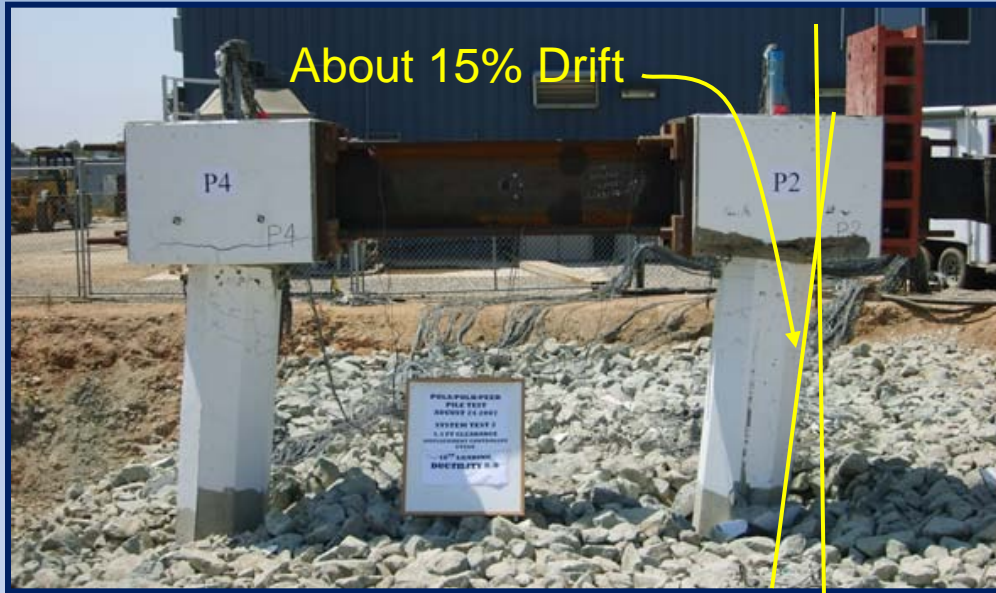


Pile Crushing  $\mu_{\Delta} = 3$  (2.4% drift)



Test Ended  $\mu_{\Delta} = 16$  (12.9% drift)

# POLA Full Scale Connection Tests (Ref. 4)



Test Frame (UCSD 2007)



Deck



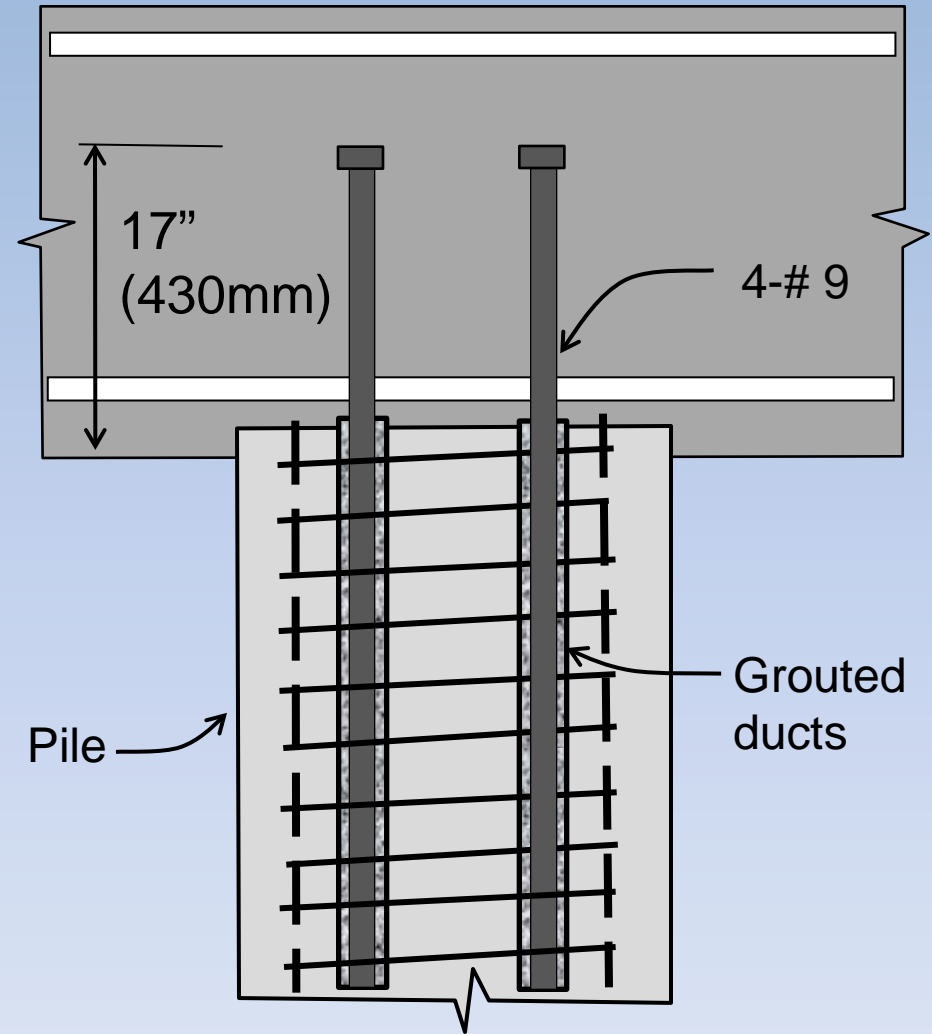
Deck Prying Spall & Interface Gap



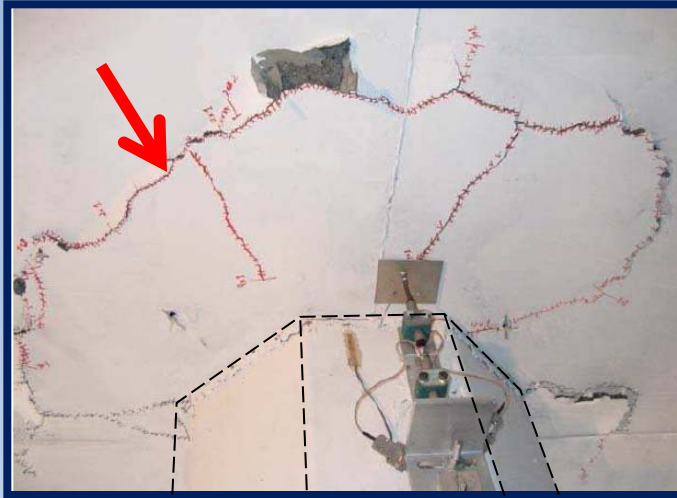
In-Ground Hinge

# POLA Secondary Seismic Pile Tests (Ref. 2,3)

- 24" deck (600mm)
- 4-# 9 headed dowels
- 16 - 0.6 in. strands



# POLA Secondary Pile Behavior (Ref. 2,3)



Deck Spalling  $\mu_{\Delta} = 3$  ( 1.5% drift)



Spall Removed  $\mu_{\Delta} = 4$  (2.3% drift)

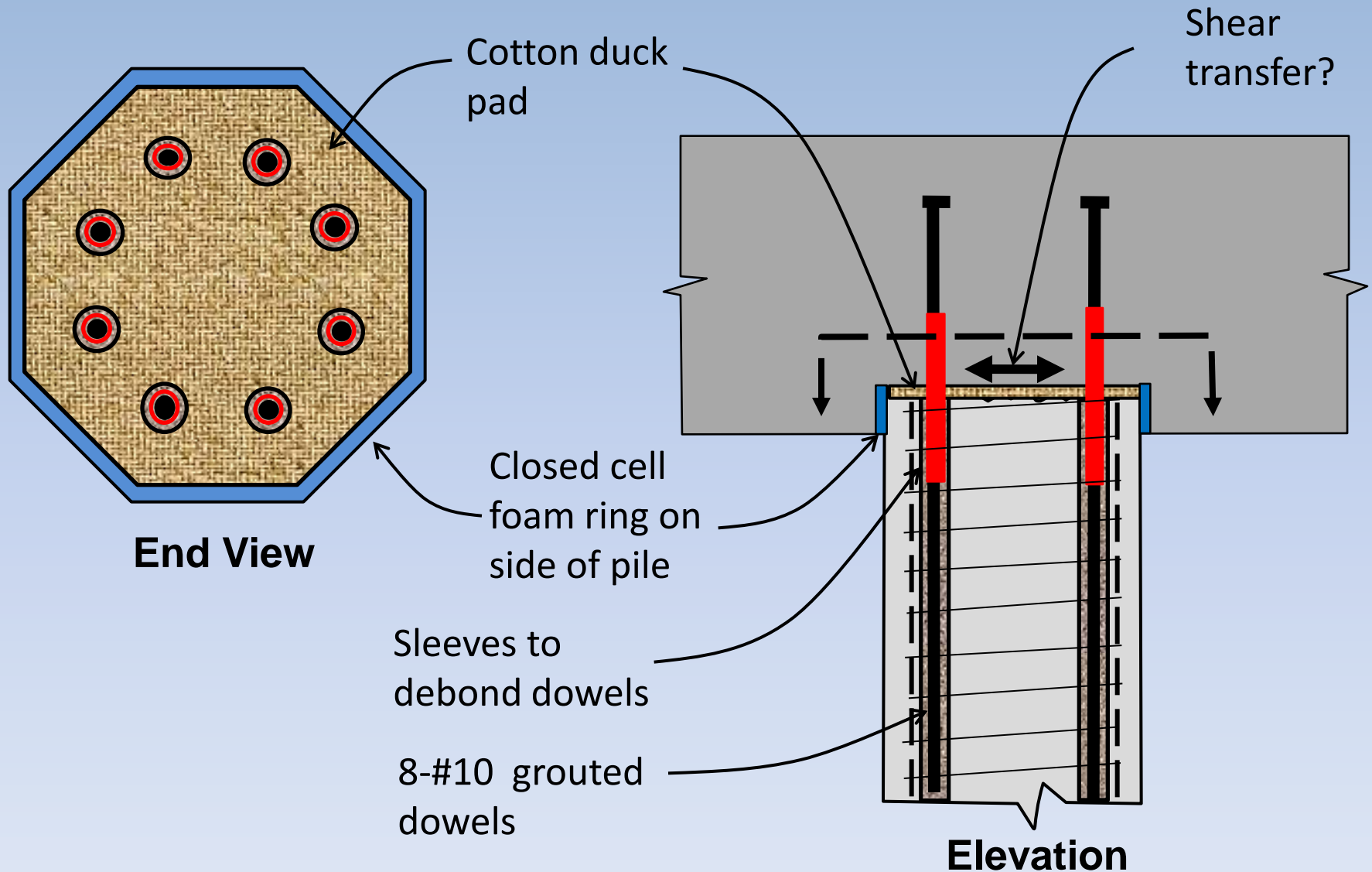


Pile Crushing  $\mu_{\Delta} = 6$  (3.4% drift)



Test Ended  $\mu_{\Delta} = 18$  (10% drift)

# 2008 UW / NEES Test on Isolated Interface (Ref. 5)



# UW / NEES Isolated Interface Behavior (Ref. 5)



**2.5% Drift**

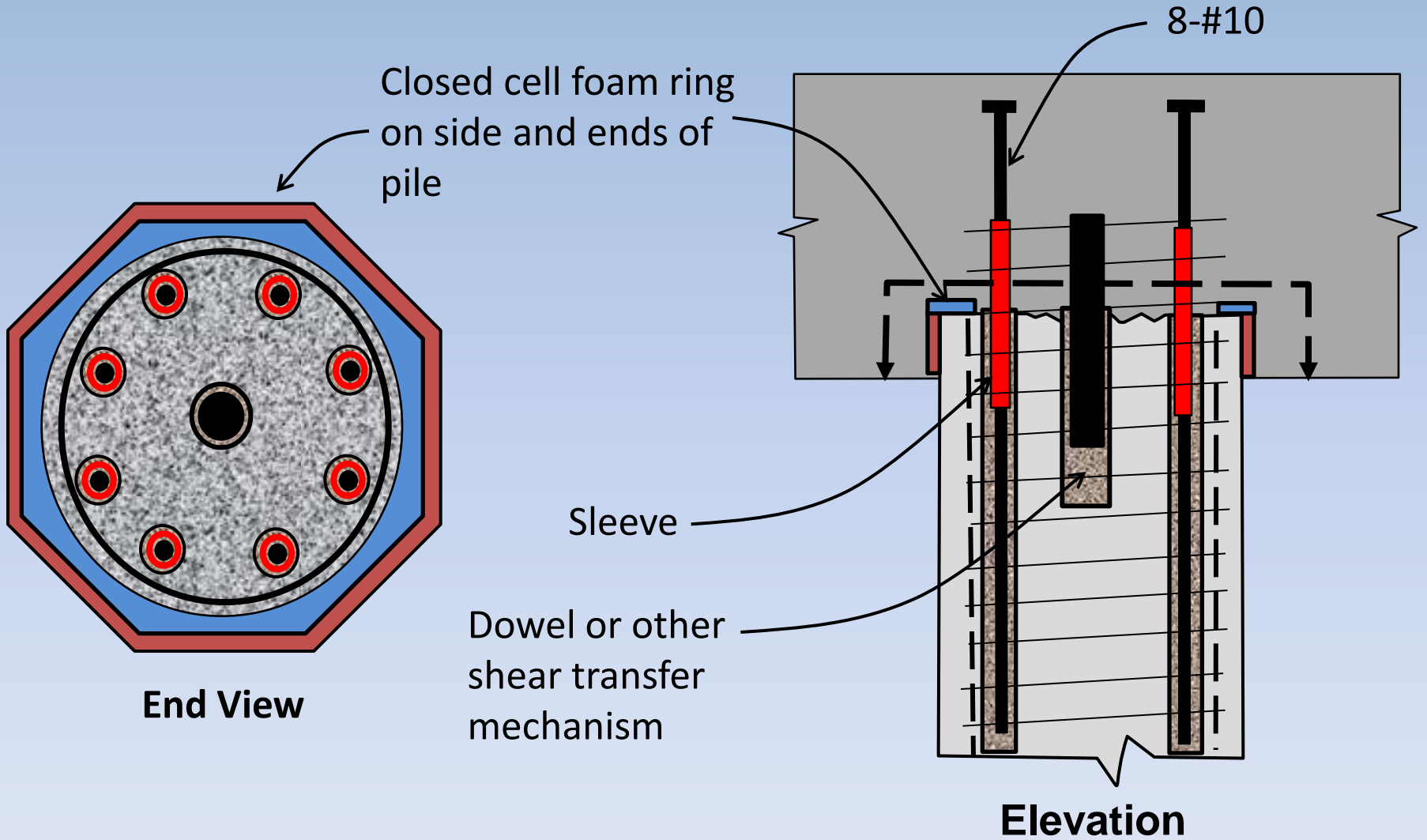


**5% Drift**



**8.4% Drift**

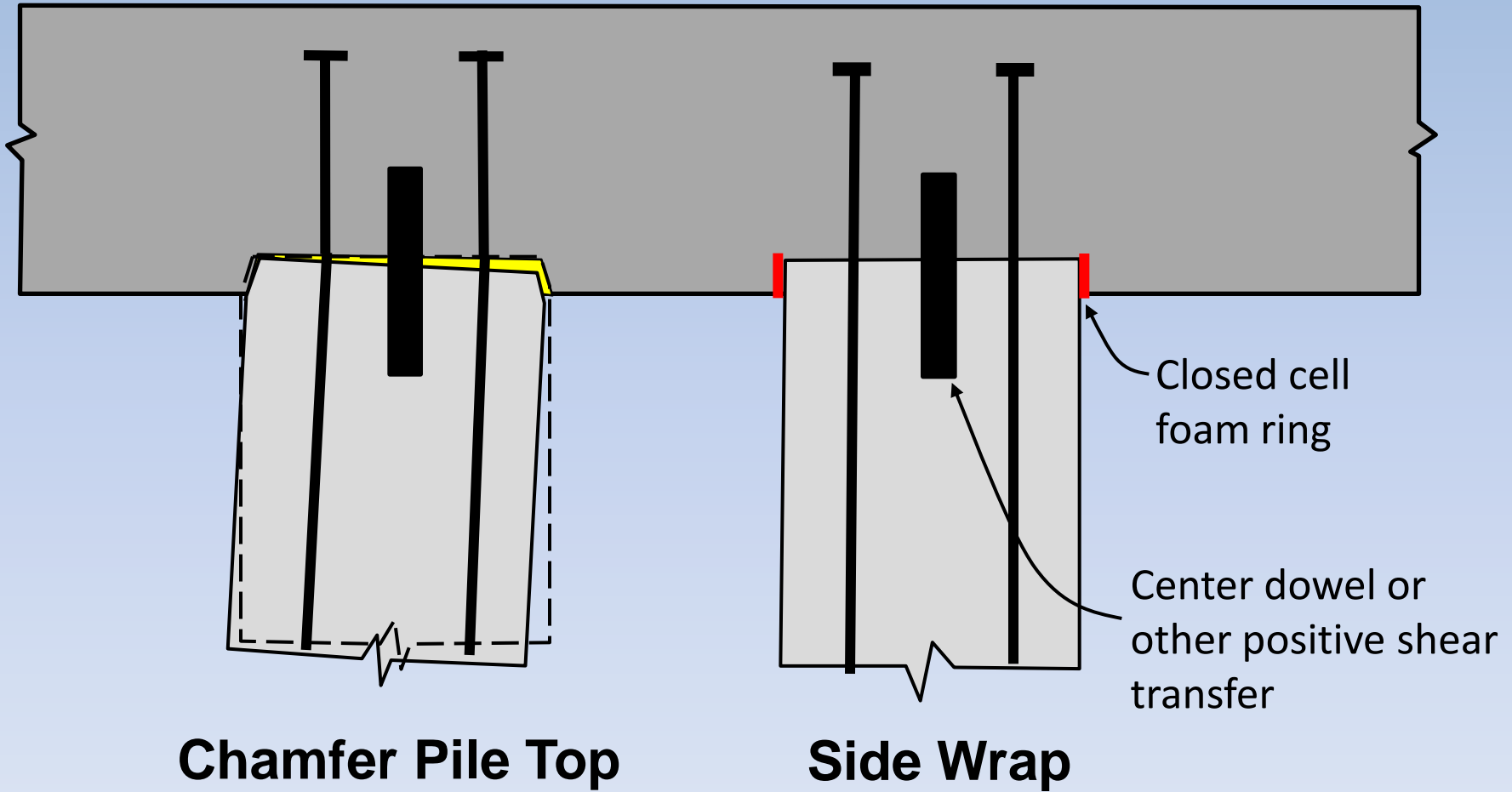
# Possible Improvement to Isolated Interface Connection (verify by testing)



# Summary of Dowelled Connection Tests

- Connections performed as expected
- Pile rocking dominated performance
- Interface gap complicates shear transfer
- Deck spalling may be preventable
- Interface isolation appears promising if positive shear transfer is provided

# Possible Methods to Minimize Deck Spalling



**Chamfer Pile Top**

**Side Wrap**

Closed cell  
foam ring

Center dowel or  
other positive shear  
transfer

# Pile Spiral Confinement Requirements

- Proposed minimum spiral requirements less than ACI 318
  - $\rho_s = 0.007$  in the ductile region
  - $\rho_s = 0.005$  outside the ductile region
- Spiral amount based on capacity versus demand analyses
  - Pile shear
  - Rotation in plastic hinge zones
  - Joint shear
- Spiral development requirements same as ACI 318.

# Presentation Summary

- Connections considered
  - Pipe piles
  - Prestressed concrete piles
- Focused on prestressed concrete piles
- Presented damage reduction strategies
- Spiral requirements summarized

# Acknowledgments

## Associates

1. Gayle Johnson
2. Tim Mays
3. COPRI committee members

## References

1. POLA Code 07, 2007, The Port of Los Angeles Seismic Code, <http://www.polaseismic.com/polacode.htm>
2. Krier, C.J., Restrepo, J. I., Blandon, C.A., 2008, Seismic Testing of a Full-Scale Pile to Deck Connections, University of California at San Diego.
3. Restrepo, J.I., Yin, P., Jaradat, O.A., Weismair, M., 2007. "Performance of New Pile to Deck Connections Under Earthquakes", Proceedings of the 2007 ASCE Ports 2007 Congress, San Diego, CA.
4. Blandon, C.A., Seismic Analysis and design of Pile Supported Wharves, Rose School, Pavia Italy, 2007.
5. Jellin, A. R., 2008, Improved Seismic Connections for Pile-Wharf Construction, University of Washington, Seattle, WA.